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# DAMAGE TO THE HIGHWAY SYSTEM FROM THE PISCO, PERU EARTHQUAKE OF AUGUST 15, 2007

by Jerome S. O'Connor, Lucero Mesa and Monique Nykamp







Technical Report MCEER-07-0021 December 10, 2007

This research was conducted at the University at Buffalo, State University of New York and was supported by the Federal Highway Administration under contract number DTFH61-98-C-00094.



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Funded principally by NSF, the State of New York and the Federal Highway Administration (FHWA), the Center derives additional support from the Department of Homeland Security (DHS)/Federal Emergency Management Agency (FEMA), other state governments, academic institutions, foreign governments and private industry.





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Jerome S. O'Connor,<sup>1</sup>Lucero Mesa<sup>2</sup> and Monique Nykamp<sup>3</sup>

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#### **Overview**

This damage assessment report is the product of a field investigation undertaken in September 2007 after the  $M_w$  8.0 August 15, 2007 Pisco, Perú earthquake. It provides a brief description of the event and the consequential damage to the highway system. It relies on government reports issued immediately after the event that include investigations from leaders in Perú's academia, and the authors' field observations. The purpose of the report is to document the performance of structures designed according to AASHTO specifications and to help assess the adequacy of the standards used at the time of construction. Another important benefit of the task is the exchange of information for the mutual benefit of engineers and policy makers here in the U.S. and in Perú.

Although there was widespread destruction of buildings in the Ica region of Perú, damage to the highway system was less severe. Traffic on the Pan-American Highway, which is the backbone of the highway system, was interrupted at numerous points but most repairs were made within a few weeks. The one exception was the Huamani Bridge, which was still closed for repair six weeks after the earthquake.

Highway infrastructure suffered some damage from shaking, but most damage resulted from earthquake induced liquefaction, which was accentuated by the earthquake's unusually long duration (>170 seconds). Liquefaction likely caused one major slope failure on the Pan-American Highway that led to 75 mm (3") wide shear cracks in a three-cell concrete box culvert. It also caused some parts of the roadway to shift laterally and the pavement to be broken up and faulted. It most likely precipitated damage to one abutment of the Huamani Bridge that was observed. This five-span bridge also had shear cracking of piers and a 100 mm (4") lateral movement of the superstructure.

There were 15 significant rockfalls that blocked roads, but most travel ways were opened again by the time of the field investigation. There were at least two instances of failed retaining walls observed by the team. At least two large bridges were hit by falling boulders, causing serious damage.

Although this report is produced in English, it contains a translation of technical terms so that it is more useful to a Spanish speaker. It also contains a set of suggestions from the authors on how to manage Perú's highway infrastructure for better performance in the future.

Both U.S. Customary (English) and SI (metric) units are used in this report. Perú uses SI units, but U.S. readers of this report may be more comfortable with English units. All dimensions noted from field observations should be considered to be approximate.

The photographs herein were taken by Jerome O'Connor unless otherwise noted.

### Acknowledgments

The post-earthquake investigation was funded by the Federal Highway Administration (FHWA) under Project DTFH61-98-C-00094, which is led by MCEER Special Tasks Director George C. Lee, Ph.D., under the direction of Phil Yen, Ph.D., FHWA Contract Officer's Technical Representative.

The authors would like to thank the organizers of the investigation: Alex Tang, L&T Engineering & Project Management Consultant, ASCE's Team Leader; Jack López Acuña, Jack López Ingenieros; Lucero Mesa; and Jack López Jara, T.Y. Lin International, Inc. in Lima Perú. Jack, in particular, is to be thanked for his diligence in arranging travel and lodging accommodations, and providing for site access and meetings with local officials. Milton Córdova Cóndor, from Condorco SRL is to be thanked for his untiring service providing safe and efficient transportation for the field inspections.

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- Carlos Valdez Velasquez-López and Juan Carlos Paz Cardenas, and others at the Ministry of Transportation (Ministerio de Transporte y Comunicaciones MTC) for reporting on their experiences dealing with the earthquake and during the recovery efforts.
- Ing. Jack López Acuña, Jack López Ingenieros S.A.C.
- Ing. Jack López Jara, T. Y. Lin International, Inc.
- Dr. Julio Kuroiwa, Professor Emeritus, National University of Engineering and Scientific Advisor to Perú's Civil Defense
- Dr. Ing. Jorge E. Alva Hurtado, Dean of the School of Engineering at Perú National University (Universidad Nacional de Ingenieria).
- Ing. Juan Pedro Andía Morón, Regional Director of Transportation and Communication for the Ica regional government.
- Ing. Niser Macedonio Quispe Arias, Ica Region, who accompanied the team for several days as they toured the region and inspected bridges and highway features.
- Manuel A. Olcese Franzero, Director of Soil Mechanics Laboratory and Daniel Quiun Wong at Catholic University of Perú (Pontificia Universidad Católica del Perú – PUCP).
- Ing. Gladys Villa Garcia, Director of Laboratory for Seismic-resistant Structures at Catholic University of Perú (Pontificia Universidad Católica del Perú PUCP.

- Other professors at Catholic University and Perú National University who shared their personal experience and documentation that will help to provide a complete account of the event and its impacts.
- Visiting professors from the University of Tokyo, Drs. Paola Mayorca and Jorgen Johansson.

El Comité Investigador del Sistema Vial agradece y quiere darle las gracias a sus anfitriones y a la gente del Perú, por su hospitalidad y su cordialidad durante su estadía en el país. Les deseamos mucha suerte en el futuro y esperamos volver un día a disfrutar de su hermoso país bajo mejores circunstancias.

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## **1.0 Introduction**

This report is the product of a field investigation undertaken about five weeks after the August 15, 2007 Pisco, Perú earthquake. The reconnaissance trip was sponsored by Federal Highway Administration under Project DTFH61-98-C-00094 being carried out by MCEER, University at Buffalo. The investigation took place September 21, 2007 through September 28, 2007.

The MCEER investigation was performed jointly with the American Society of Civil Engineers (ASCE)'s lifeline's field reconnaissance, led by Alex Tang, P.E. A subgroup of the ASCE team (Lucero Mesa and Monique Nykamp) joined Jerome O'Connor to form the highway system investigation team (see figure 1-1). The team was assisted by the firm Jack López Ingenieros, S.A.C. and others in gathering preliminary information and scheduling meetings with local authorities. The Acknowledgment section of this report recognizes individuals and agencies that provided assistance and Appendix A describes the experience of the team members.



*Figure 1-1. Highway System Investigation Team: J. O'Connor, M. Nykamp and L. Mesa* 

The purpose of the investigation was two-fold:

*Data Collection:* to observe and document damage to the highway system from ground shaking, soil instability, liquefaction, lateral spreading, landslides, and rock falls. This data will lead to a better understanding of the threats to the highway system and the behavior of bridges designed according to American Association of State Highway and Transportation Officials (AASHTO) specifications (used in Perú). It can enhance our understanding of the loads imposed by hazards, the response of structures, conditions that affect a structure's ability to resist those loads (e.g., the presence of scour and or corrosion), and the presence of more than one load effect.

*Information Sharing:* to publish information and disseminate to others in the U.S. and abroad for our mutual benefit. During meetings with the Perú Ministry of Transportation, the government expressed a desire to see the final report. Sharing knowledge and technical expertise with our neighbors to the south will foster better international relationships and help develop personal contacts in countries that are exposed to the same hazards as we are.

At the suggestion of one transportation official in Perú, this report contains an Appendix C with a translation of technical terms. Although not as beneficial as a complete English-to-Spanish translation of this report, it will aid Spanish speakers in the comprehension of its content. The terms will also be useful to English speakers who wish to better understand engineering terms used in Perú.

### 2.0 Background

The country of Perú has a land mass of 496,000 square miles and is situated on the west coast of South America. The capital Lima is located at S. Lat. 12° - W. Long. 77°. Ica Department, where the effect of the earthquake was most severe, is approximately four hours south of Lima (S.Lat. 14° - W. Long. 75.5°). The cities of Pisco and Ica are 236 km (147 mi.) and 303 km (188 mi.) south of Lima, respectively. The country has a population of 28 million, who mostly live in Lima.

The country of Perú is divided into 24 departments, which are analogous to states in the U.S. The departments are divided into provinces, which are similar to counties in the U.S., and finally, small governmental entities, districts.

The earthquake was felt in much of the country but damage was most evident in the Ica Department, where over 688,000 people reside (see table 2-1)<sup>1</sup>. Within the Ica Department, the cities of Pisco and Ica were most affected.

Province	(Number of Political districts)	Population in 2002
Chincha	(11)	177,000
Pisco	(8)	127,000
Ica Provinc	re (14)	304,000
Palpa	(5)	18,000
Nasca	(5)	62,000
	Total	688,000

Table 2-1. Population of Ica Department

The county's climate is affected by its coastal frontage and the Andes Mountains. Along the coast there are arid deserts and sand dunes, whereas in the eastern part of the country, there is rough terrain with abundant water from mountain streams. Rainfall along the coast in the Ica region is typically less than 15 mm ( $\sim 1/2$ ") per year.

Perú has a long history of earthquakes. In 1650, a strong earthquake destroyed the city of Cusco, which was the capital of the Incan empire that stretched from what is now Chile to Colombia. In 1664, an earthquake devastated the city of Ica. As recently as 2001, a  $M_w$  8.4 earthquake struck off the coast.<sup>2</sup> Most of the earthquakes are subduction zone earthquakes caused by subduction of the Nasca plate under the South American Plate.

<sup>&</sup>lt;sup>1</sup> *Ica Perú Tourist Guide* (2002). 2<sup>nd</sup> Edition, Slyt Grafic E.I.R.L. Ica, Perú.

<sup>&</sup>lt;sup>2</sup> ASCE Atico, Perú Earthquake Monograph No. 23.

The Ica region also suffers from occasional El Niño rains and subsequent flooding. Ica was devastated by flooding of the Ica River in 1963, then again, in 1983 and 1998, when rains from El Niño resulted in inundation of 80% of the city.

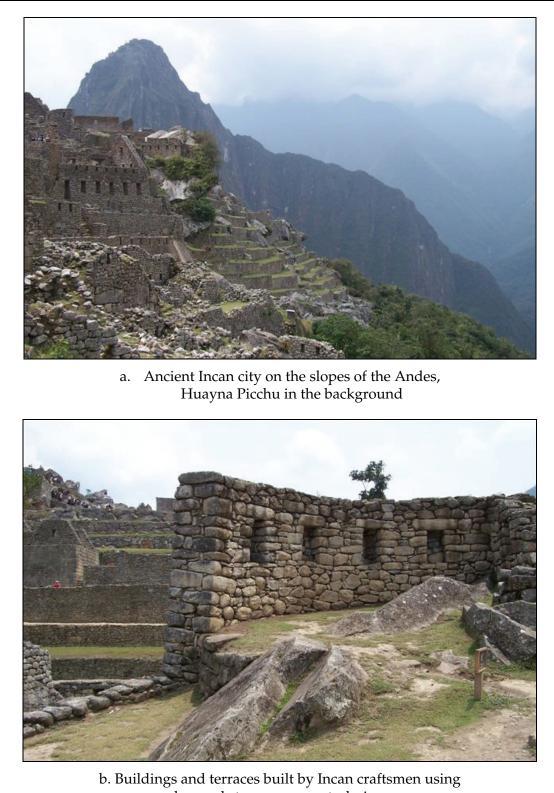
Figure 2-1 shows the location of Perú, and the location of the earthquakes' epicenter.

The soils near the coast consist largely of liquefiable silts and sand. Where firm ground exists near Lima, it is an alluvial deposit locally known as "Lima conglomerate," a dense, slightly cemented gravel. In the Ica/Piso area, fine soils are cemented eolian deposits. Away from the coast, the ground is very rocky with very little arable soil.



Figure 2-1. Map of South America and Perú with Epicenter of the August 15, 2007 Pisco Earthquake

No report on Perú is complete without some mention of Machu Picchu (see figure 2-2). At 2,430 m (7,970 ft), this pre-Columbian site is in the "highlands" of the Andes and considered one of the seven new wonders of the world. Fortunately, the ancient city was not damaged by the Pisco earthquake.



advanced stone masonry techniques

Figure 2-2. Incan Capital of Machu Picchu

## 3.0 The Highway System

The Ministry of Transportation and Communication (MTC) is the administrative branch of government with responsibility for the highway system in Perú. The MTC was gracious enough to meet with the investigation team at their offices in Lima to give an overview of the consequences of the earthquake.

Their explanation started with a description of the road network. There are three classes of roadways in Perú (refer to table 3-1 and figure 3-1).

Classification	Type of Road	Km in Ica
National Highway System (Nacional)	Primary	527,300 (100% is paved)
Departmental (Departamental)	Secondary	342,440 (25% is paved)
Rural (Vecinal)	Rural	1,488,490 (4% is paved)

Table 3-1. Road Classifications in Perú<sup>3</sup>

Gobierno Regional de Ica 2004

*National:* The primary highways are part of the National Highway System (NHS). The backbone of the NHS is the Pan-American Highway (Carretera Panamericana Sur, also known as Route 1S) the country's major North-South route. The NHS roads are run by concession. This means that the national government has a long-term (30-year) contract with a private company that serves as an operator and collects tolls in exchange for maintaining the roads in good condition. In the event of an earthquake, the contractor has a contractual obligation to restore the highway system to its original condition. Other routes on the NHS are the E-W routes Los Libertadores (Route 24), and San Juan-Nazca-Cusco (Route 26)

*Departmental:* Departmental (regional) roadways are secondary roads, and because of the nature of the terrain, are often unpaved. The Ica Departmental system includes Routes 100 (i.e., Chincha Alta-Huachinga-Huanchos Highway), 102 (Chincha-Tambo de Mora), 106 (Los Aquijes-Huambo), 107 (Tulin-Agua Perdida-Ayacucho), 108 (El Engenio-Huarasaca), 109 (Muelle Acari-EMP R1S km 518), and 110 (Ica-Parcona-Tinguina-Los Molinos-Tambillos).

*Rural:* Rural roads (caminos vecinales) are lightly traveled, farm-to-market roads, typically dirt.

<sup>&</sup>lt;sup>3</sup> Plan Vial Departamental Participativo (2004) Gobierno Regional de Ica, Perú

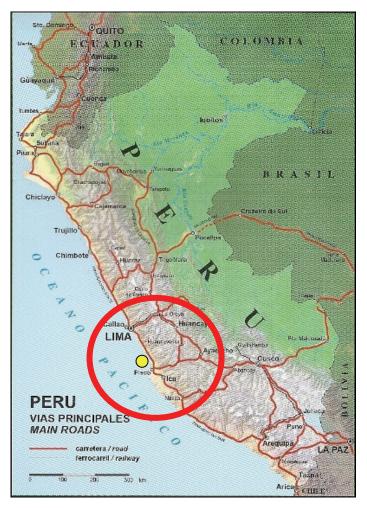
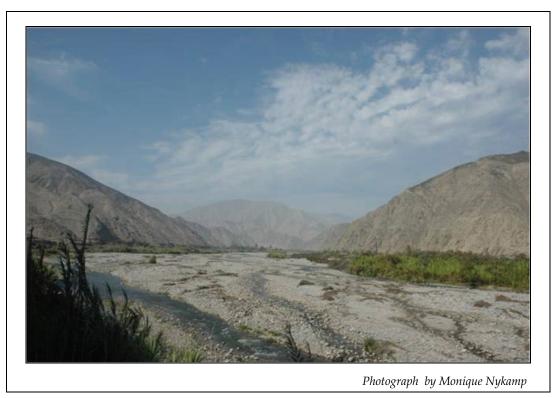


Figure 3-1. Peru's highway system with area of interest noted.

There are four major rivers in Ica. All carry rain water and snow melt west from the Andes Mountains toward the Pacific Ocean. The rivers are Rio Chincha (or San Juan), Rio Pisco, Rio Ica, and Rio Grande. In 1998, during the El Niño, these rivers carried great flood waters and well as a debris flow known as *huayco (or huaico)*. During the 2007 reconnaissance, the water level in the river was very low but the riverbeds were wide, indicating a history of occasional heavy flooding (see figure 3-2). As one travels east (i.e., upstream) there is more water and vegetation in an otherwise barren region.

Since bridges are a vital yet vulnerable link of the highway infrastructure, much of the investigation focused on the performance of bridges. In Perú, a structure is considered a bridge if the span is greater than 10 m (32.8'). (See table 3-2 for definitions used in Perú to describe structures carrying roads over water.) According to records in the Ica Department, there are 31 bridges, totaling approximately 850 m (2,789') in length.



*Figure 3-2.* The wide riverbed of Rio San Juan is indicative of the large volume of water that the river carries in years when El Niño returns.

English Term	Spanish Term	Span Length (meters)
Bridge	Puentes	> 10 m
Culverts	Pontones	1m < span < 10m
Pipes	Alcantarillados	<1m

Table 3-2. Drainage Structure Classifications Used in Perú

Perú relies on AASHTO specifications for the design of its bridges.<sup>4</sup> The 475-year event found in the Standard Specification for Highway Bridges has typically been used. The team observed bridges with wide bridge seats and transverse restraints such as concrete shear blocks. Piles or caissons are not typically used for foundations, apparently due to a of lack of necessary equipment.

The building code<sup>5</sup> in Perú divides the country into three seismic zones (see figure 3-3), for both building and bridge design. Peak ground accelerations (PGA) specified for each zone are indicated.

<sup>&</sup>lt;sup>4</sup> AASHTO

<sup>&</sup>lt;sup>5</sup> Perú, Ministry of Housing, Construction and Sanitation, NTE E.30, 2003

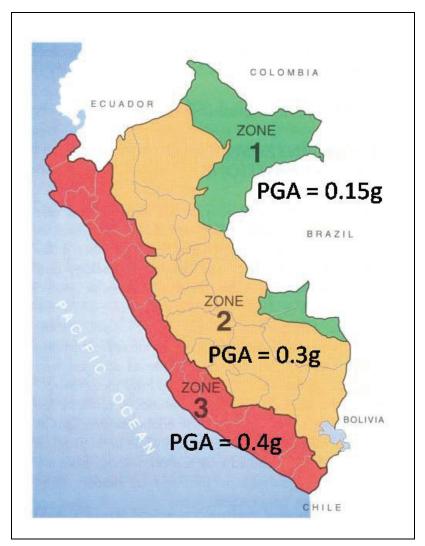


Image courtesy of Dr. Julio Kuroiwa<sup>6</sup>

Figure 3-3. Seismic hazard zones in Perú's design code

<sup>&</sup>lt;sup>6</sup> Kuroiwa, Julio (2004). DISASTER REDUCTION. Living in Harmony with nature, Quebecor World Perú S.A, ISBN 9972-9999-0-4.

### 4.0 The Earthquake of August 15, 2007

Perú sits on a subduction zone where the oceanic Nazca Plate is moving under the South American continental plate (see figure 4-1). On August 15, 2007, an earthquake occurred at 6:41 pm local time (23:41 GMT).<sup>7</sup> The epicenter of the earthquake was located at S. Lat. 13.4° - W. Long. 76.5°. This is 50 km (31 miles) W-NW of Chincha Alta, Perú or 150 km (93 miles) SSE of Lima. The length of this subduction was reportedly 150 km (93 miles).

The earthquake resulted in at least 519 deaths and 1,874 injuries (National Civil Defense Institute (INDECI). In addition, 54,926 buildings were destroyed and 20,958 were seriously damaged. In the city of Pisco, over 80% of the buildings were destroyed or damaged.

The earthquake's moment magnitude  $(M_w)$  was 8.0, producing the greatest ground motion in the Ica region where the Modified Mercalli Intensity (MMI) was VIII. Shaking was also very strong in Lima (MMI=V) and was felt 1,100 km away in the Andes highlands city of Cusco (MMI=III). There were reports of some damage to buildings in Lima, and a small tsunami was generated near Lagunilla, on the south coast of the Paracas peninsula.

The ground motions of the earthquake were recorded at about 16 stations in Perú. Most of the strong motion stations are located in the Lima area, about 170 km north of the north end of the fault rupture zone. Only two ground motion recording stations are present within the fault rupture zone. These two stations are located in the town of Ica, near the south end of the rupture zone.

Peak ground accelerations (PGA) as high as 0.49 g (approximately half the acceleration due to gravity) were recorded on accelerometers at the PCN station in Parcona, Ica. This is higher than the PGA specified in the building code for seismic zone 3. Time history graphs of the earthquake show two distinct phases of strong ground motion (see figure 4-2). The earthquake had an unusually long duration, resulting in shaking that lasted >170 seconds (measured at the PUCP seismograph station).

The primary geotechnical impacts of the earthquake were due to the phenomenon of liquefaction. Liquefaction occurs during ground shaking in loose, saturated, sandy soil when the water pressure in the pore spaces increases to a level that is sufficient to separate the soil grains from each other. This phenomenon results in a reduction of the shear strength of the soil (a quicksand-like condition). Liquefaction can result in ground settlement, lateral spreading, landsliding, localized ground disruptions from sand boils (ejection of sand and water at the ground surface), and reduced vertical and lateral capacity for structure foundations. Buildings, bridges, and other structures founded on or in the liquefied soils may settle, tilt, move laterally, or collapse.

<sup>&</sup>lt;sup>7</sup> USGS: http://earthquake.usgs.gov/eqcenter/recenteqsww/Quakes/us2007gbcv.php

Because much of the affected area consists of fine grained, water saturated sediments, the geotechnical aspects of the earthquake were most dramatic. There was evidence of widespread liquefaction, especially along the coastal regions where the water table was not far from the surface. Newspapers had accounts of water spouting 2 m (6.5') up from the ground during the earthquake in Ica. Even near Lima where the PGA's were less than 0.1 g, there were utility poles tipped from liquefaction of the supporting soils.

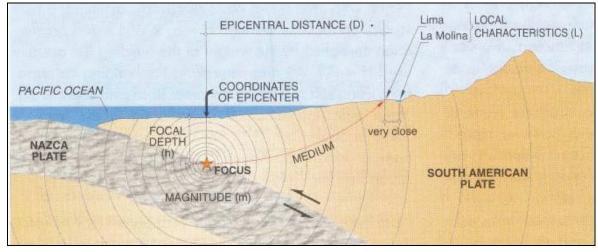
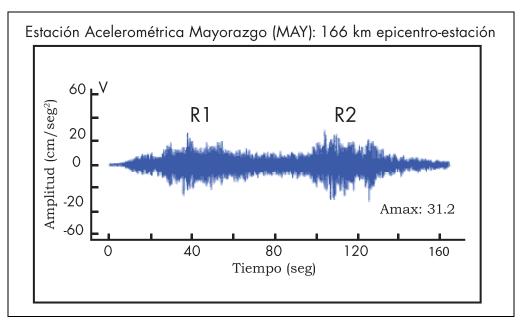


Image courtesy of Dr. Julio Kuroiwa

Figure 4-1. Illustration of the Subduction Process



*Figure 4-2. Time history with two distinct peaks and a very long duration*<sup>8</sup>

<sup>&</sup>lt;sup>8</sup> Tavera, H., Bernal, I., Salas, H. (2007). *El Sismo de Pisco del 15 de Agosto, 2007 (7.9Mw),* Dirección de Sismología – CNDG.

#### 5.0 Damage to the Highway System

The highway system reconnaissance team was charged with assessing the extent of damage to roads and bridges due to the earthquake. Field investigation began after conferring with the MTC in Lima, professors from Catholic University and National University who had completed preliminary investigations, and local officials in the Department of Ica. This assisted greatly in prioritizing investigative efforts, and understanding the types of problems that resulted from the earthquake.

MTC reported that essentially all highway damage that occurred was in the Ica Department, the region closest to the epicenter of the earthquake. Therefore, the investigation was conducted in this region. Routes surveyed include the Pan-American Highway (on the National Highway System), Ica Departmental Route 100, Ica Departmental Route 110, National Route 24, and local streets carried by bridges within the City of Ica (see figure 5-1).

Although there was widespread destruction of buildings in Ica, structural damage to the highway system was less severe. Most damage resulted from geotechnical issues. Along the coast, liquefaction was induced by the presence of loose, sandy soils, a high ground water table, and the unusually long duration of motion. This resulted in slope failures, lateral spreading, shifting of the pavement as well as failure of buried culverts. There were 15 significant rockfalls that blocked other roads but most were opened again by the time of the field investigation. There were at least two instances of failed retaining walls observed by the team, and at least two large bridges were hit by falling boulders, causing serious damage.

Approximately twenty-four sites were inspected. This includes bridge structures and other highway features such as culverts, roadways, embankments, and retaining walls. 62% of these sites had evidence of damage. The MTC was not able to provide an estimated cost of damage to the highway system and the investigators did not try to quantify the value of the damaged facilities.

A brief summary of the findings is presented in table 5-1. The site number was assigned according to the sequence of the field inspections. The "km" column is the distance along the highway as indicated by road markers.

Site No. / Name	Highway	Coordinates	km	Damage	Comment
1	Pan-American Highway	S13°19'44" W76°14'00"	182	Evidence of cracking in cut- slope parallel to roadway	Lateral spread.
2	Pan-American Highway		185	Distortion and cracking of pavement	Caused by liquefaction. Repaired.
3	Pan-American Highway	S13°23'42" W76°11'52"	188	Cracks from lateral spread along 200m length of road. One meter (3') lateral displacement of edge line.	Caused by liquefaction.
4	Pan-American Highway	S13°24'48" W76°11'20"	190	Embankment failure & shear failure of 3-cell concrete box culvert.	Road repaired. Culvert shored. Caused by liquefaction/ lateral spread.
5 Puente Huamani over Rio Pisco	Pan-American Highway	S13°41′13" W76°09′31"	224	Shear cracking in piers. Tipped abutment. 100mm (4") lateral movement of superstructure halted by shear blocks on piers.	Closed for repair. Traffic is being detoured over riverbed with pipes carrying low flow water.
6 Puente Aylloque	Route 100	S13°24′13" W75°57′03"		Minor cracking of abutment.	1-span 10m (33') timber bridge is subjected to overloads.
7 Puente over Rio San Juan	Route 100	S13°23′57" W75°56′24"	27	none	2-span timber bridge is subjected to overloads.
8 Puente Huachinga over Rio San Juan	Route 100	S13°22′08" W75°50′60"	39	Severe damage to lower chord of 1-lane steel truss from boulder impact. Cracked welds at gusset plates; Severe erosion at abutment.	Evidence of previous impact damage to bottom chord, either from rockfall or El Niño debris flow (huayco).
	Route 100		27	Failure of retaining wall.	
9 Puente Ica over Rio Ica	Pan-American Highway (Ave. Los Maestros)	S14°05′25" W75°43′11"	305	none	3-span CIP; 29m (95')
10 Puente over Rio Ica	Ave. Cutero in Ica City			none	2-span CIP; 31m (100')
11 Puente Grau	Ave. in Ica City			none	1-span concrete;25m (80')

Table 5-1.	Summary	of Field	Inspection	Findings
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Site No. / Name	Highway	Coordinates	km	Damage	Comment
12 Puente Socorro	Ave. in Ica City	S14°03′20" W75°43′36"		none	3-span CIP; 33m (110')
13 Puente over Canal	Route 110	S13°58'48" W75°41'53"		none	1-span CIP; 12m (40')
14 Puente over Canal	Route 110	S13°57'40" W75°41'39"		none	1-span CIP; 10m (33')
15	Route 110		14	Failed retaining wall.	
16 Puente Los Molinos over Rio Ica	Route 110	S13°55′29" W75°40′43"		Three significant shear cracks in P/S beams at supports; Lateral displacement, Tipped pier from previous scour.	5-span, 1-lane, non redundant 2- girder P/S; Built in 1932.
17 Puente Quinga over Rio Pisco	Route 24	S13°38'27" W75°43'06"	54	Rockfall may have initiated failure of rock wingwall. Previous erosion at wingwalls.	36.5m (120') P/S; Posted for 60T max.
18 Suspension bridge over Rio Pisco	Pedestrian trail, not on highway system.	S13°41′42" W75°48′38"		none	91m (300') Suspension ped bridge built 2001.
19 Puente Toro over canal	Route 24			none	2-lane concrete bridge 18m, (60')
20	Road to Pisco from Pan- American Highway at km 233.	S13°42'47" W76°09'18"		Liquefaction resulted in faulted pavement & N-S cracking.	Broken watermain.
21	Just off Pan- American Highway on road to Pisco.	S13°42'47" W76°09'18"		Damage to pavement due to liquefaction; ruptured waterline.	Excavated for paleoliquefac- tion study.
22	Pan-American Highway	S13°36′27" W76°09′01"	218	Road damage due to liquefaction; tipped poles.	Pavement has been repaired.
23 Puente Cruz Verde	In Tambo de Mora.			Minor(1 inch) wingwall displacement.	Newer bridge.
24	Road to the beach at Sunampe.	S13°25′54" W76°10′56"		Vertical shift as great as 10' due to liquefaction.	Road repaired; escarpment, sand boils, cracking are evident in the vicinity

Note: If data was not readily available, the cell was left blank.

In Ica, the team was able to conduct a damage assessment of the highway system by traveling five routes in a 4x4 vehicle (see figure 5-1):

- Pan-American Highway
- Departmental Route No. 100 (Carretera Chincha)
- Departmental Route No. 110
- National Route No. 24 (Los Libertadores)
- Local Streets in the city of Ica

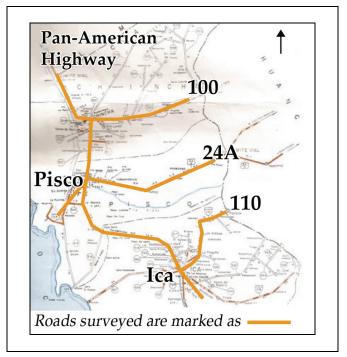


Figure 5-1. Ica Roads Surveyed During Damage Assessment of the Highway System

What follows is a description of the findings along each route. Photographs are used extensively to illustrate the conditions that were found. In cases where repair of damaged features had already taken place, photographs of the condition immediately following the earthquake are provided courtesy of the team's in-country contacts. The findings are presented in same order that the routes were traveled.

There were a few bridges that were not able to be inspected because of time limitations. To provide a complete picture of the situation, additional pictures are added that were provided by others.

#### 5.1 Pan-American Highway

On September 25, 2007 the highway system investigation team drove south on this main North-South route from Lima to the Department (state) of Ica, where earthquake damage was most evident. In the Ica region, it is a two lane road, paved with asphalt. Locations along the Pan-American Highway are conveniently posted as the number of kilometers (km) south of the capital Lima.

The Pan-American Highway parallels the coastline, and as such, is built upon sandy and silty soils. The terrain is coastal desert with large sand dunes and sparse vegetation. The Pacific Ocean is as close as 100 m (328') to the west. The road is bordered on the east by large dunes consisting of cemented sand formations (see figure 5-2).



*Figure 5-2. East shoulder of Pan-American Highway at km 178 pushed up onto sand dune after lateral displacement of roadway during liquefaction and "sloshing" of supporting ground.* 

At numerous stops along the highway, the team encountered evidence of earthquakeinduced liquefaction and lateral spreading. Lateral spreading occurs on gentle slopes as a result of soil liquefaction. When the soil liquefies, gravity causes the land to move downslope.

Conversations with individuals that had conducted inspections immediately following the earthquake recounted stories from locals stating that the water table was high prior to the event. Acceleration records of the event show that there were ground vibrations for an extended period (over 170 seconds). This ground motion, soil amplification and liquefaction resulted in damage to the pavement, embankment and cut slopes, drainage structures and shoulders, as evident from the photographs in figures 5-2 through 5-26. The photographs are in order based on a journey in a southerly direction from Lima. Sites referenced are listed in table 5-1.



*Figure 5-3. Sand boils such as this are left behind as evidence of liquefaction in the area. This one was discovered at km178 near Jajuay Beach (Playa Jahuay)* 



*Figure 5-4.* Note the white edge line. This is evidence of lateral displacement of the roadway at km 188 due to liquefaction at the toe of slope and reflection of seismic ground waves against the more rigid sand dune on the east side of the highway.



*Figure 5-5. Fissure along the northbound road shoulder at km 188 resulting from lateral spread away from the more rigid sand dunes in the left of the picture (Tom Cooper inspects). The ocean is out of view on the right.* 



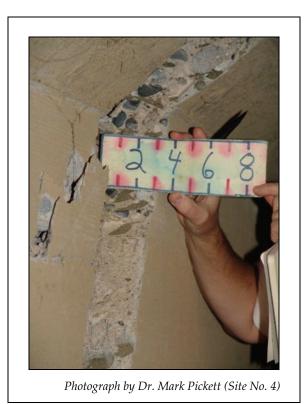
*Figure 5-6.* Embankment and roadway failure at km 190. The picture was taken facing north. Note the Pacific Ocean in the top left of the photo. The damage was caused by amplification of the ground shaking. Liquefaction of the wet coastal silts and sands led to lateral spreading and the embankment failure.



*Figure 5-7. View from the Pan-American Highway looking West toward the Pacific Ocean. Notice the cracking of the earth from liquefaction and lateral spreading that led to the massive embankment failure at km190* 



*Figure 5-8. Outlet end of failed box culvert at km 190.* 



*Figure 5-9. Shear failure resulted in a 3-inch wide crack in the three-celled concrete box culvert at km 190* 



*Figure 5-10. Timber shoring and a horizontal crack in a concrete box culvert km* 190.



*Figure 5-11. Pan-American Highway near San Clemete Approximately km 220.* 



Figure 5-12. Pan-American Highway near San Clemente Approximately km 220.



Figure 5-13. Pan-American Highway Near San Clemente Approximately km 220.



Figure 5-14. Pan-American Highway near San Clemente Approximately km 220.



*Figure 5-15.* This is one of the many sections of the Pan-American Highway that needed pavement repairs after the earthquake. Paving was still going on six weeks after the event.

#### 5.1.1 Huamani Bridge

Huamani Bridge (Puente Huamani) is a significant link of the highway system that was taken out of service by the earthquake. It is located at km 224 of the Pan-American Highway and is located directly east of the epicenter. This concrete structure was built in 1950 to cross the Rio Pisco (Pisco River) to join the city of Pisco with the village of San Clemente to the north. It is 136 meters (446') long and two lanes wide (6.7 m or 22'). The bridge was designed to an H-15 AASHTO loading and had been posted with a 36 ton weight limit.

Although all five spans of the bridge remain standing, there was evidence of liquefaction in the sandy soils and evidence of strong shaking (see figures 5-16 through 5-26). The abutments and piers are built on massive walls, similar to caissons but are not supported by piles. According to record plans obtained by Ing. Jack López Jara, the 4 m (13.1') high pier stems rest on solid concrete caissons that are founded 7.8 m (25.6') into the stream bed. There was no indication of tipping or settlement of the piers.

There was cracking of the top of the pier caps, concrete diaphram and shear blocks, apparently resulting from lateral movement of the superstructure. The second pier from the south end had particularly severe damage from this lateral loading. The fact that the superstructure was only moderately displaced is evidence of the successful use of shear blocks.

The south abutment tipped toward the stream as a result of liquefaction of the sandy soil at that end of the bridge. During the inspection, the superstructure was temporarily supported and the south abutment was being rehabilitated. The north abutment is on a more competent material (a lightweight siltstone) that did not liquefy. There was only hairline cracking of that abutment.

Immediately after the earthquake, the bridge was reopened with traffic restricted to just one lane. Shortly thereafter, however, it was closed to all traffic as a precautionary measure and so that repair work could be accomplished. The bridge was under repair at the time of the inspection. Bridge damage can be summarized as:

- Severe cracking of one pier stem
- Severe cracking of transverse concrete diaphragms
- Minor to severe cracking and spalling of horizontal shear blocks and pier caps (see figure 5-20)
- Tipping of the south abutment toward the stream bed (approximately 100 mm (4") at the top)
- Lateral displacement of the superstructure (approximately 100 mm or 4")
- Damage to approach pavement, especially at south end of the bridge

Had the bridge not been part of a lifeline transportation route, this performance could be considered acceptable. The bridge did not collapse and was successful in preventing loss of life. The damage it suffered, though severe, is repairable.

Prior to the earthquake, the government had already been planning to build a new bridge at this site within the next few years. A few suggestions are provided in Appendix B of this report.

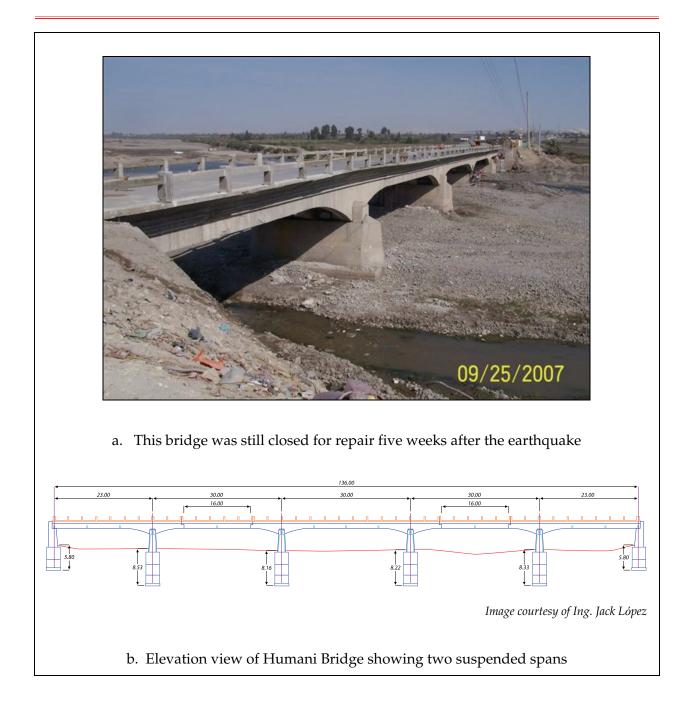
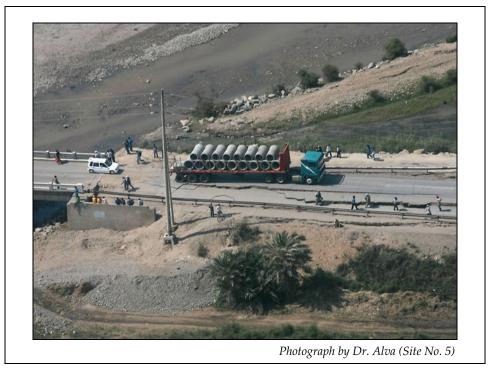


Figure 5-16. Five-span Humani Bridge on the Pan-American Highway at km 224



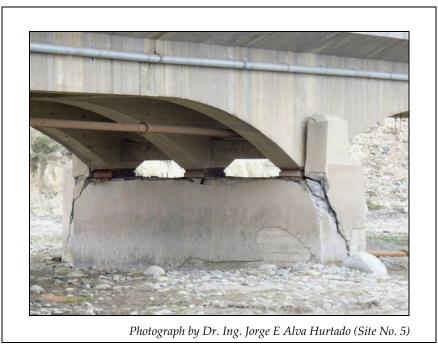
*Figure 5-17. Pavement damage at the approach to the Huamani Bridge caused by soil liquefaction and lateral spread of embankments.* 



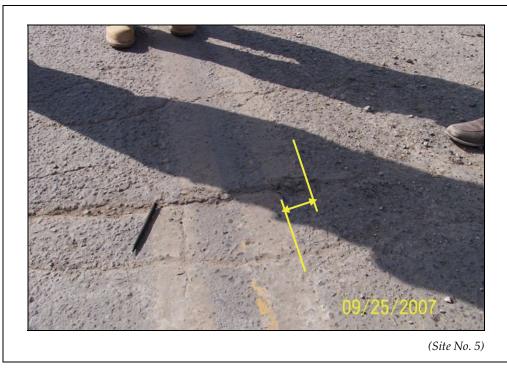
*Figure 5-18. Approach pavement at the south abutment of Huamani Bridge has settled dramatically due to liquefaction of supporting soils and lateral spread of the embankment.* 



*Figure* 5-19. *Sand boils in the stream bed at the Huamani* Bridge *indicate that the soil liquefied.* 



*Figure 5-20. Severe cracking of Pier 2 (from South end) necessitates extensive repair but the horizontal shear blocks managed to retrain lateral movement of the superstructure. This pier has roller expansion bearings.* 



*Figure 5-21. Horizontal displacement of the superstructure is evident by the shift in the centerline at a joint* 



Figure 5-22. Cracked deck due to horizontal movement of the span.



*Figure 5-23.* Broken railing and shift in curbline resulting from lateral movement of the span. Corrosion probably played a role in failure of the barrier.



*Figure 5-24. Shear failure of pier due to lateral loading imposed by the superstructure. Note that this pier has roller expansion bearings.* 



*Figure 5-25. Crack at end of concrete beam due to horizontal loading at a pier with fixed bearings.* 



a. South end of span is supported while tipped abutment is replaced



b. Ongoing reconstruction of South Abutment

(Site No. 5)

Figure 5-26. Puente Huamani over Rio Pisco

#### 5.2 Departmental Route No. 100 (Carretera Chincha)

Departmental Route 100 is a secondary road on the Ica Departmental Road Network (see figures 5-1 and 5-27 through 5-38). It starts at the Pan-American Highway in Chincha and heads in an easterly direction. This is an unpaved road with no appurtenances such as signage or guiderailing. Km posts denote the distance away from Chincha. As one travels east, the terrain becomes more and more rugged. The road winds around the rocky foothills of the Andes mountains and generally follows the San Juan River, which is also known as the Chincha River. The river carries water down from the Andes mountains towards the ocean but most of the time the volume of water is very low. By the time it gets to the ocean, the water seeps underground and the river is almost dry. Flooding, however, does occasionally occur. The El Niño effect, which last occurred in 1998, has left a very wide riverbed as its legacy (see figure 3-28).



Figure 5-27. Unpaved Departmental Route 100 in Ica.



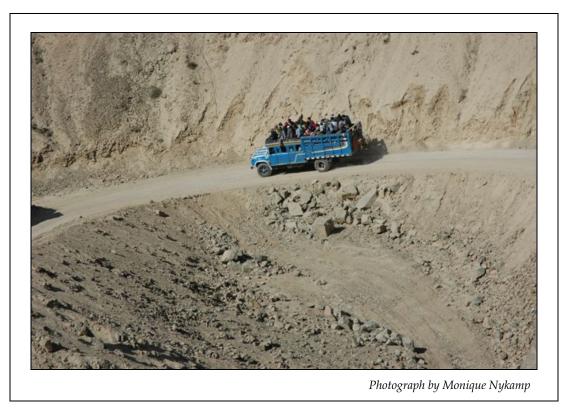
*Figure 5-28. Typical terrain along Departmental Route 100. The road tracks the mountain on the left and is subject to rockfalls as is seen in this photo.* 



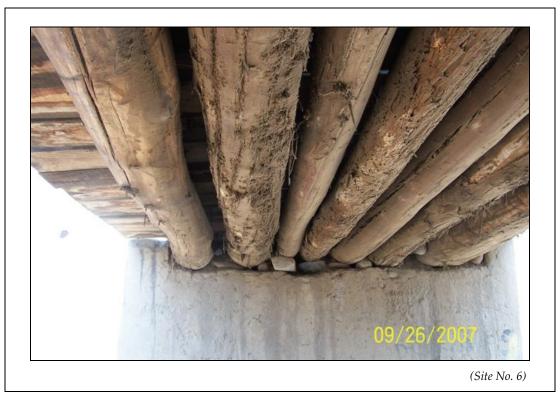
*Figure 5-29. Monique Nykamp and remains of several rockfalls that blocked Departmental Route 100 at km 29.* 



Figure 5-30 Damage on Route 100 at km 27



*Figure 5-31. Heavily packed passenger bus illustrates the number of people counting on having Route 100 passable.* 



*Figure 5-32. Being relatively light, this one-span log bridge fared well in the earthquake.* 



*Figure 5-33. Slight cracking may have been from horizontal loads imposed by the superstructure.* 



*Figure 5-34.* This log bridge could be seen deflecting under heavy truckloads in this rural area. It is much more likely to fail from overload than earthquake.



Figure 5-35. Puente Huachinga at km 39 built in 1966.



Figure 5-36. Puente Huachinga at km 39. This steel truss bridge has suffered severe damage to the bottom chord from presumably previous debris impact (top arrow). There is a large granite boulder jammed between the two channels of the bottom chord that has fallen from the adjacent mountain from this earthquake (bottom arrow).



Figure 5-37. Puente Huachinga at km 39. It is uncertain whether this bottom chord damage was caused by falling boulders from this event or whether it remains from a previous hazard, such as El Niño effect flooding that last occurred in 1998. This deformation can limit the capacity of this tension member and put additional stress in the undamaged channel. Figure 5-38 shows a close-up of the crack that the lower arrow is pointing to.



*Figure 5-38. Cracked weld along bottom chord resulting from distortion of the channel after impact from debris. There is potential for this type of crack to propagate into the primary member and cause sudden failure.* 

#### 5.3 Departmental Route No. 110

The most significant finding on route 110 was at the site of the Puente Los Molinos over the Rio Ica (see figures 5-1 and 5-39 through 5-46). This is a five span, one lane bridge that was built in 1932. The superstructure is structurally non-redundant, consisting of two cast-in-place concrete girders.

The bridge may have been more vulnerable to earthquake damage because of unrepaired damage stemming from the flooding that accompanied the 1998 El Niño. Scour at Pier No. 1 exceeds two meters and Pier No. 2 is tipped. There is a noticeable kink in the alignment of the bridge, partially because of the tipped pier but possibly because of a lateral shift from the recent earthquake. According to a local resident, "it was like that before but worse now."

At three different locations, there are shear cracks that appear to be new. Although they could be a result of truck loading, they appear to be from a lateral stress to the girders applied at the supports. Although there was not excessive displacement, this type of damage is consistent with earthquake induced lateral loading.

Evidence of liquefaction was noted at this bridge by several sand boils (see figure 5-43). Liquefaction of the alluvial soils in the riverbed may have also contributed to observed damage to the bridge. Liquefaction likely resulted in reduced lateral support to the bridge piers.

Although the bridge could be considered fragile because of the previous scour and the newer cracking of the structural members, it is a vital link in the transportation system. During the inspection, several fully loaded trucks crossed the bridge hauling out rubble from nearby towns.



*Figure 5-39. Puente Los Molinos. Note the pre-existing scour (>2m or 6') at Pier No. 1 (as identified from left to right) (see arrow) and tipped Pier No. 2 shown with the arrow on the right.* 



Figure 5-40. Tipped pier resulting from El Niño flooding of 1998.



a. What appears to be a fresh shear crack in concrete beam at Pier No. 1 pedestal

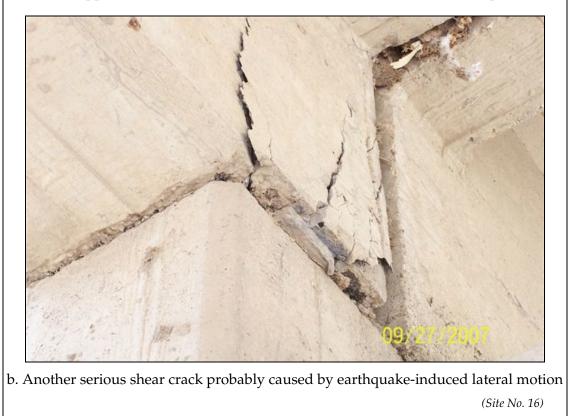


Figure 5-41. Puente Los Molinos over Rio Ica



Photograph by Lucero Mesa (Site No. 16)

*Figure 5-42. Two girder, non redundant, CIP concrete superstructure.* 



*Figure 5-43. The scour monitoring device painted red is an example of good bridge management practice.* 



*Figure 5-44. Monique Nykamp pointing to sand boil as evidence of liquefaction in river bed of Rio Ica at Puente Los Molinos.* 



*Figure 5-45. Lateral shift of the superstructure.* 



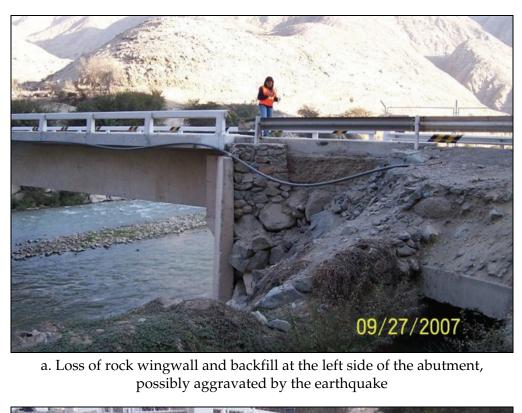
*Figure 5-46. Heavy trucks continue to rely on the Los Molinos bridge to haul debris from damaged villages nearby.* 

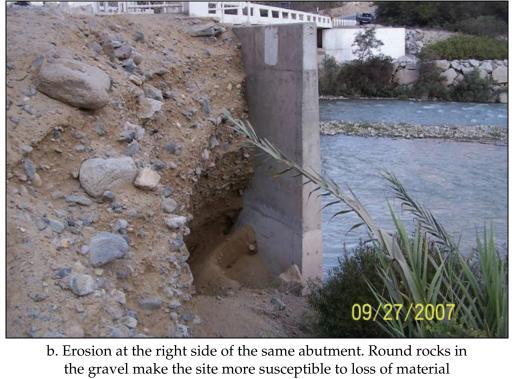
### 5.4 National Route No. 24 (Los Libertadores)

Route 24 is a two lane paved highway that runs from the Pan-American Highway at San Clemente easterly toward the mountains. Although there are several bridges along this route, no significant damage was discovered (figures 5-47 through 5-49).



*Figure 5-47. The rocky terrain above Puente Quinga presents a danger of impact from boulders. The bridge showed evidence of slight lateral motion that resulted in minor cracking of the approach curbing.* 





(Site No. 17)

Figure 5-48. Puente Quinga over Rio Pisco



Figure 5-49. This example of newer bridge construction performed well.

## 5.5 Local Streets in the City of Ica

Several streets and bridges were inspected within the City of Ica. Although the team saw numerous adobe buildings that had failed, the bridges did not show any evidence of damage. The bridges inspected were cast-in-place concrete built within the past twenty years, so the type of construction probably played an important part in their good performance.



*Figure 5-50. Concrete bridge near the epicenter that performed well.* 



*Figure 5-51. Concrete bridges like the Puente Socorro within the City of Ica performed well.* 

### 5.6 Other Significant Sites

Due to time constraints, the highway system inspection team was not able to traverse every road in the affected area. Through reports provided by the MTC, the team was able to focus on the most significant damage, reported earlier in this report. This section provides additional information about performance of features that were not inspected personally (figures 5-52 through 5-58).



Figure 5-52. Geologist Dr. Carlos Costa, UNSL in Argentina pointing out soil layers within cross section of sand boil during paleoliquefaction study. He was working with Patricio Valderrama, Perú Geological Survey.

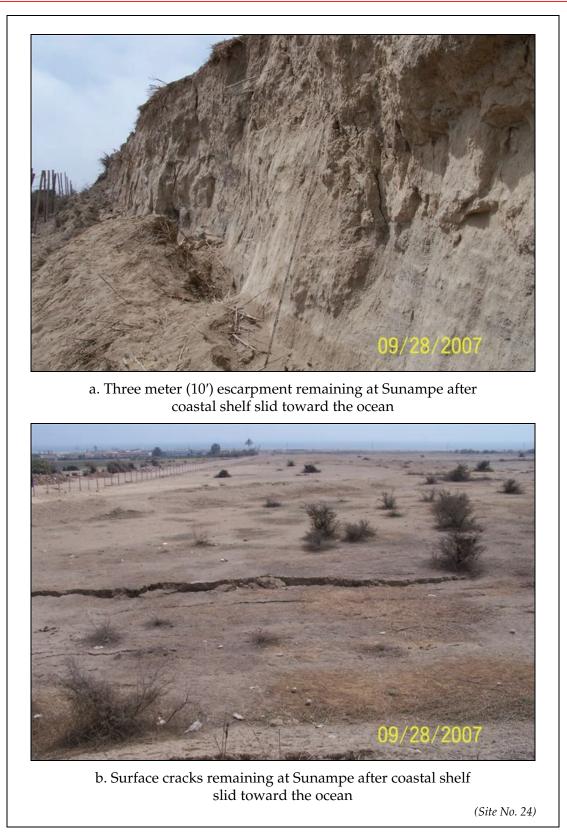
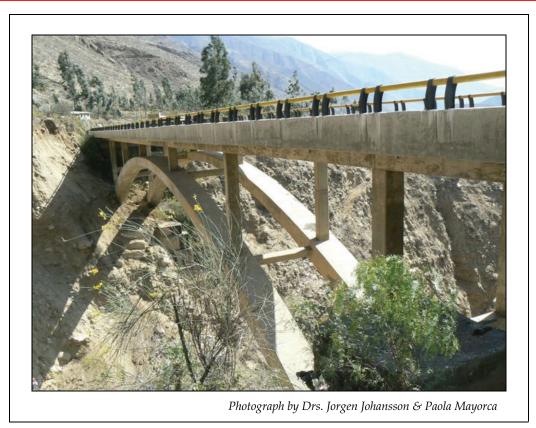
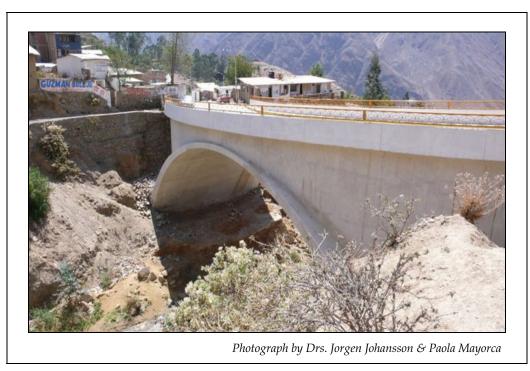


Figure 5-53. Road to the beach at Sunampe.



*Figure 5-54. Huaytara Bridge TsejTsi – no reported damage.* 



*Figure 5-55. Huaytara Bridge Tranca II – no reported damage.* 



*Figure 5-56.* This photo is indicative of the extensive disruption of an urban street in Pisco. It is Calle Independencia.



Figure 5-57. Damage from a tsunami that swept up on Paracas.



*Figure 5-58. In the mountainous Andean highlands, several pedestrian suspension bridges survived the earthquake without damage.* 

# 6.0 Summary

The  $M_w$  8.0 earthquake that struck on August 15, 2007 has been labeled the Pisco Earthquake. The highway system investigation team visited 24 sites in the state of Ica, Perú to assess the performance of roads and bridges that were subjected to ground motion with a duration as long as 170 seconds and accelerations as great as 0.49g. Perú's building code specifies a PGA of 0.4 for construction in Seismic Zone 3 (reference figure 3-3).

The Pan-American Highway, which tracks the coastline, is built on liquefiable soil and suffered damage. Considering the intensity and duration of shaking, damage to bridges was minimal. The Huamani Bridge suffered the greatest damage and was still closed for repair six weeks after the event.

Observed damage included:

- Rockfalls at fifteen or more locations. Although most roads had been cleared of rock debris, there were still numerous locations where fractured, unstable blocks of rock above the roadway pose a hazard.
- Liquefaction of sandy/silty soils leading to tipping of utility poles, and tipping of a bridge abutment (Puente Huamani). It also initiated lateral spread that displaced an entire section of roadway, sheared a three-cell concrete box culvert, caused an embankment slope failure, and severe cracking and faulting of asphalt pavement.
- Two or more were bridges damaged by boulders falling from an adjacent mountain (Puente Huachinga and one other in the north reported by MTC).
- Cracking of concrete pier caps and shear blocks (Puente Huamani). The shear blocks, designed to provide lateral resistance to the bridge superstructure, successfully restrained the bridge so that damage is repairable.
- Shear cracking of concrete girders at supports (Puente Los Molinos). This appears to have been from lateral movement of the superstructure.
- Lateral displacement of bridge superstructures (about 100 mm or 4") (Puente Huamani and Puente Los Molinos).

# 7.0 Additional Resources

American Association of State and Highway Transportation Officials, (2002), *AASHTO Standard Specifications for Highway Bridges*, 17th Edition, 01-Sep-2002, 1028 pages, ISBN: 1560511710.

AASHTO Subcommittee on Bridges and Structures website <u>http://bridges.transportation.org/?siteid=34</u>

Barrionuevo, A. and Puertas, L. *New York Times*, Peru Family's Struggle Reflects Hardships After Quake, 8/22/07, Vol. 156, Issue 54044, pages A3.

Buckle, I.G. (Coordinating Author), Friedland, I., Mander, J., Martin, G., Nutt, R. and Power, M. (2006), *Seismic Retrofitting Manual for Highway Structures: Part 1 – Bridges*, MCEER-06-SP10, MCEER, University at Buffalo, State University of New York, 656 p.

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Perú Transportation and Communications, Ministerio de Transportes & Comunicaciones (MTC), <u>http://www.mtc.gob.pe/portal/flashes.html</u>

Power, M. (Coordinating Author), Fishman, K., Makdisi, F., Musser, S. Richards, R. and Youd, T.L. (2006), *Seismic Retrofitting Manual for Highway Structures: Part 2 – Retaining Structures, Slopes, Tunnels, Culverts and Roadways*, MCEER-06-SP11, MCEER, University at Buffalo, State University of New York, 370 p.

Universidad Nacional de Ingeniería Facultad de Ingenieria Civil, Centro Peruano Japonés de Investigaciones Sísmicas y Mitigación de Desastres - CISMID, website: <u>http://www.cismid-uni.org</u>

Wartman, J., Rodriguez-Marek, A., Repetto, P. C., Keefer, D.K., Rondinel, E., Zegarra-Pellane, J., and Baures, D. (2003). "Ground Failure," paper from *Southern Peru Earthquake of 23 June 2001 Reconnaissance Report, Earthquake Spectra, Supplement A* to Volume 19, January 2003, pages 35-56.

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## **Appendix A: Team Member Biographies**



#### Jerome S. O'Connor (Team Leader)

Jerome is Senior Program Officer for Transportation Research with the MCEER at the University at Buffalo. He earned his Bachelors and Masters Degrees in Civil Engineering from Rensselaer Polytechnic Institute. During his 20 years with the New York State Department of Transportation, he led a 24-member team in Bridge Management, Inspection and Vulnerability Assessments and became known as a champion of fiber reinforced polymer composites for bridge applications. At MCEER, he manages a \$13.8M research project for the Federal Highway Administration

(FHWA) with the objective of reducing the vulnerability of our nation's highway infrastructure to damage from earthquakes and improving "multiple hazard" performance. One week after Hurricane Katrina in 2005, he led a bridge damage reconnaissance team supported by the National Science Foundation and FHWA, and a month later joined a Lifelines Team for the Department of Commerce's National Institute of Standards and Technology.

He serves on FHWA's Virtual Team for Earthquake Engineering. He has spoken and written internationally on the subject of vulnerability assessment. Past honors include an Award of *Meritorious Service* by NY's Department of Civil Service, selection as NYSATE statewide "Engineer of the Year," and project leader for the Civil Engineering Research Foundation's 2000 Charles Pankow Award for Innovative Applications. He is a Fellow of the American Society of Civil Engineers.



#### Lucero Mesa

Lucero Mesa is a senior bridge engineer and the team leader for the Seismic Engineering Support Group at the South Carolina Department of Transportation, (SCDOT). She earned her Bachelors Degree in Civil Engineering from the University del Valle, in Cali, Colombia. Lucero worked several years in Venezuela before moving to the United States where she earned her Master of Science in Civil Engineering from the University of South Carolina. During her 18 years with SCDOT, she has worked on

numerous bridge projects, including the Arthur Ravenel Jr. cable stayed bridge over the Cooper River in Charleston, South Carolina, which is the longest span cable stayed bridge in North America. As part of this bridge project, she worked very closely with the Seismic Resource Panel of experts that advised SCDOT on the seismic issues.

Lucero's major area of interest has been earthquake engineering as applied to transportation infrastructure, and the ways in which other hazards affect bridge

performance. She led the development and implementation of the SCDOT Seismic Design Specifications for Highway Bridges in 2001, and the Probabilistic Seismic Hazard Maps research project for seismic analysis in 2002.

Lucero has served as chair of several committees and tasks forces. She is currently serving as chair of the Pile to Pile-Cap Connection research project for SCDOT (2007-2009), and Technical Committee co-chair of the 6<sup>th</sup> National Seismic Conference (6NSC) to be held in Charleston, South Carolina in July, 2008. She is also a member of the Transportation Research Board Seismic committee and the Geo-Seismic Subcommittee, the Federal Highway Administration Virtual Team for Earthquake Engineering, ASCE Technical Council on Lifeline Earthquake Engineering, and ASCE Infrastructure Champion for Region 4. She served on the AASHTO Task Force 193 that reviewed the new seismic guidelines, and she is currently serving on the LRFD Seismic Guide Specification Improvement Team (SGSIT).



### Monique A. Nykamp, P.E.

Monique Nykamp, P.E. is a geotechnical engineer with Shannon & Wilson, Inc., Seattle, Washington. She earned her Bachelors degree in Civil Engineering and Masters degree in Geotechnical Engineering from University of California at Berkeley.

Monique Nykamp has about 17 years of experience at Shannon & Wilson, Inc. She has worked on numerous domestic and international geotechnical projects involving bridges,

embankments, buildings, piers, retaining walls, landslides, dams, roadways, shoring systems, and excavations for a variety of clients. Her field experience includes coordinating and supervising subsurface explorations, performing and monitoring numerous types of field tests, installing and reading instrumentation, performing condition surveys, and monitoring construction activities. Monique has served as project engineer and project manager for a variety of large and small projects giving her extensive experience in shallow and deep foundation design, slope and embankment stability and settlement studies, liquefaction evaluations, and retaining wall and shoring design.

Ms. Nykamp has authored several publications on topics such as foundations, shoring, compaction grouting, liquefaction and lateral spreading. She is active in the American Society of Civil Engineers and serves on the Technical Council on Lifeline Earthquake Engineering.

## **Appendix B: Recommendations**

The main body of this investigative report provides factual information primarily from observations. In contrast, this appendix lists subjective recommendations based on the authors' collective experience. It comes from a request from a local transportation engineer to reciprocate this assistance with something specific that would help them. For instance, they asked for technology transfer assistance. In response, the authors have provided copies of manuals, URL addresses of informative websites, the translation of technical terms found in Appendix C and the following advice. The points offered below may help improve system performance, reduce risk of failure, and generally better manage highway infrastructure. The authors' realize that lack of resources may prevent full adoption of these suggestions, but list them for consideration regardless.

- 1) Establish a bridge management system
  - a) Create a detailed bridge inventory in digital format that is available to managers and inspectors. This could start as a simple spreadsheet in Microsoft Excel. Contact the authors for information to track.
  - b) Assign a unique bridge number to each bridge and record information such as: precise longitude and latitude, length, orientation, structure type, materials, year built, length, width, etc.
  - c) Create a bridge history folder for each bridge and maintain a copy locally. Include a set of the original design, construction drawings, as-built drawings, a record of any changes that have been made (e.g., maintenance, repairs or retrofits).
  - d) Create maps with precise location of bridges.
  - e) Perform an engineering analysis to determine the actual load capacity of each bridge in its current condition. Update after any changed condition.
  - f) Post bridges with a weight restriction that is based on calculations and enforce the posting.
- 2) Establish a bridge inspection program
  - a) Conduct a thorough baseline inspection of all existing bridges and large culverts.
  - b) Monitor the condition of each bridge by having them inspected regularly by a trained engineer (e.g., every two years). The objective is to monitor any changes so deficiencies can be corrected before it results in a catastrophic collapse or gets very costly to fix.
  - c) Conduct a thorough inspection of bridges after any major event such as an earthquake or flood. Monitor bridges closely during a flood so that it can be closed before a failure becomes imminent.
  - d) Institute a "flagging" system whereby bridge inspectors issue a red flag for structural conditions that need immediate attention, a yellow flag for lesser structural concerns, and a safety flag for things that are non-structural but potentially unsafe.

- 3) Establish a bridge repair and maintenance program
  - a) Address conditions that could lead to failure of a bridge.
  - b) Repair erosion and scour locations before they get worse.
  - c) Retrofit bridges to correct existing deficiencies or to strengthen them to carry loads they are being subjected to.
- 4) To mitigate rockfall and landslides:
  - a) Bring down precarious rock overhangs where practical.
  - b) Flatten cut and fill slopes (rock 1.5:1, compacted fill 2:1)
  - c) Always compact fill material.
  - d) Consider steel catch fences, draperies, and rock protection embankments when practical and cost effective.
- 5) For construction of new bridges:
  - a) If at all possible, avoid damage from flooding, local scour, liquefaction, tsunami, huayco, etc. by choosing an alignment and/or elevation that minimizes the risk. If an ideal location is not feasible, take appropriate measures to protect the bridge by using piles, deep foundations, etc.
  - b) Conduct a subsurface investigation by taking borings at the proposed location of each substructure unit.
  - c) Assess liquefaction potential of the local soils.
  - d) Avoid building bridges on silty or sandy soils without piles.
  - e) Design a deep foundation using drilled shafts, prestressed piles.
  - f) Minimize the number of piers in the water
  - g) Raise the elevation of the bridge deck to protect from flooding, huayco, or tsunami.
  - h) Provide scour protection.
  - i) Make the superstructure continuous over piers.
  - j) Utilize wide bridge seats.
  - k) Utilize shear blocks to restrict lateral movement.
  - l) Include a detailed plan for routine inspection and maintenance
  - m) Consider incorporating a monitoring system on the bridge. For instance, install accelerometers on the bridge.
- 6) Provide training to staff or consultant engineers
  - a) How to assess the severity of damage to bridges.
  - b) Advanced training on seismic design (including seismic performance, service and damage levels), fracture critical design, fatigue, etc.
  - c) Assign at least one structural engineer to each Department office to oversee technical matters related to bridges.
- 7) Use web sites such as <u>http://mceer.buffalo.edu</u> to obtain recent information on seismic design and retrofit.

- 8) Contact FHWA's International office for possible future assistance and collaboration.
- 9) Assign a point of contact for future dialogue with FHWA and other U.S. engineers.

Steps that can be taken immediately by the authors to assist officials in Perú are listed below. This communication has already been initiated and several resources have been provided.

- Share CD's with manuals in electronic format (easiest way to carry to them).
- Provide websites for technical information (so they can research what's available to them).
- Send manuals by ground mail (e.g., basic bridge inspection manuals with pictures).
- Translate technical terms to facilitate their use of material that is written in English (or provide a version of the report in Spanish).
- Provide any technical material that may be available in Spanish.

# **Appendix C: Translations**

## **English-Spanish Translation**

The following vocabulary is provided to facilitate use of this document and facilitate communication with professional counterparts in Spanish speaking countries. It includes technical terms that are pertinent to transportation engineering, bridge inspection, and geotechnical engineering. It also contains a few general terms that may be useful in an earthquake reconnaissance mission.

English	Spanish
abutment	estribo, contrafuerte
aftershock	réplica
aggregate / crushed stone for concrete	grava
American Association of State Highway and	AASHTO
Transportation Officials (AASHTO)	
angular rock	grava, roca con ángulos
asphalt	asfalto
ATM (Automated Teller Machine)	cajero automático
be careful	tenga cuidado
beach	playa
boundary	límite
bridge	puente (in Perú, >10m)
bump	jiba, chichón
channel	canal
CIP (cast in place concrete)	concreto fundido en sitio
closed	cerrado
collapsed bridge	puente colapsado
compaction	compactación
concrete	concreto
conglomerate	conglomerado
construction project	proyecto de construcción
county	provincia
crack	grieta
cross section	sección transversal
culvert	pontón (in Perú,>3m but <10m)
cut	corte
damage	daño
debris flow	huayco
debris flow (mud flow with large rocks)	huaico
deck	losa
design	diseño
detour	desvío
displaced	desplazado
earthquake	terremoto / sismo

English	Spanish
edge line / fog line	linea del borde, linea en el pavimento, linea
	de la niebla
elevation	elevación
epicenter	epicentro
excavator	excavadora
fill	relleno
flat	plano
flood	inundación
fog	neblina, niebla
foot (feet)	pie (pies) (1 metro ~ 3.1 feet (3.1')
freeway / expressway	autopista
girder	viga
GIS (Geographic Information System)	SIG (Sistema de Información Geográfica)
GPS (global positioning system)	Sistema de Posicionamiento Global
ground level	superficie del terreno
guiderail / guardrail	barandilla de metálico, riel metálico
gutter	cuneta
heavy equipment	equipo pesado
height	cota
highway	carretera
kilometers	kilómetros
landslide	deslizamiento de terreno, derrumbamiento
lateral spread	movimiento lateral
latitude	latitud
length	longitud
liquefaction	licuacíon / licuefacción
longitude	longitud
maintenance	mantenimiento
MTC (Ministry of Transportation &	MTC (Ministerio de Transporte y
Communication	Comunicaciones)
overweight	sobrecarga
P/S (pre-stressed concrete)	concreto pretensado
painted line	línea pintada
paved road	carretera asfaltada o pavimentada
pavement	pavimento
pier / bent	pilar
pile	pilote
pongo, barranco, hendidura profunda	ravine
railroad	ferrocarril
rain	lluvia
retaining wall	muro de contención
retrofit	actualizar, modernizarse
river	rio
road	via, carretera
rock	roca

English	Spanish
rock fall or rock slide	derrumbe
rock slide	desprendimiento de roca
rounded river rock, boulder	canto rodado
route	ruta
rubble, debris	escombros
sand	arena
sand boil / sand cone	volcancito
scour	erosión
secondary road	carretera secundaria
shoulder	hombrillo
sidewalk	calzada/ande'n/acera
silt	limo (lodo)
slope	talud
small gravel / pea gravel	gravita / gravilla
soft soil (ground)	suelos blandos
soil (ground)	suelo (terreno)
span	luz, ojos
speed limit	velocidad límite
stability	estabilidad
state	departamento
store / shop	tienda /almace'n
strong	fuerte, resistente
swale ( in road in lieu of a bridge)	balén
toll road	carretera con caseta de peaje
tremor	temblor
truck	camión
undermining	socavación
unpaved road	carretera afirmada
water level	nivel de agua
weight limit	carga máxima / peso máximo / tonaje límite
width	ancho
wingwall	pantalla

### Spanish-English Translation (Traducción Español-Inglés de términos técnicos)

El propósito del siguiente vocabulario es facilitar el uso de este documento y hacer más fácil la comunicación con nuestros colegas ingenieros en los países de habla Hispana. El documento incluye términos técnicos relacionados con ingeniería de transporte, inspección de puentes y geotecnia.

También contiene términos generales que pueden ser útiles en una misión de reconocimiento después de un terremoto.

Español	Inglés
AASHTO	American Association of State Highway and
	Transportation Officials (AASHTO)
actualizar, modernizarse	retrofit
ancho	width
arena	sand
asfalto	asphalt
autopista	freeway / expressway
balén	swale ( in road in lieu of a bridge)
barandilla de metálico, riel metálico	guiderail / guardrail
cajero automático	ATM (Automated Teller Machine)
calzada/ande'n/acera	sidewalk
camión	truck
canal	channel
canto rodado	rounded river rock, boulder
carga máxima / peso máximo / tonaje límite	weight limit
carretera	highway
carretera afirmada	unpaved road
carretera asfaltada o pavimentada	paved road
carretera con caseta de peaje	toll road
carretera secundaria	secondary road
cerrado	closed
compactación	compaction
concreto	concrete
concreto fundido en sitio	CIP (cast in place concrete)
concreto pretensado	P/S (pre-stressed concrete)
conglomerado	conglomerate
corte	cut
cota	height
cuneta	gutter
daño	damage
departamento	state
derrumbe	rock fall or rock slide
deslizamiento de terreno, derrumbamiento	landslide

Español	Inglesa
desplazado	displaced
desprendimiento de roca	rock slide
desvío	detour
diseño	
elevación	design elevation
epicentro	epicenter
equipo pesado	heavy equipment
erosión	scour
escombros	rubble, debris
estabilidad	stability
estribo, contrafuerte	abutment
excavadora	excavator
ferrocarril	railroad
fuerte, resistente	strong
grava	aggregate / crushed stone for concrete
grava, roca con ángulos	angular rock
gravita / gravilla	small gravel / pea gravel
grieta	crack
hombrillo	shoulder
huaico	debris flow (mud flow with large rocks)
huayco	debris flow
inundación	flood
jiba, chichón	bump
kilómetros	kilometers
latitud	latitude
licuacíon / licuefacción	liquefaction
límite	boundary
limo (lodo)	silt
linea del borde, linea en el pavimento, linea de la niebla	edge line / fog line
línea pintada	painted line
lluvia	rain
longitud	length
longitud	longitude
losa	deck
luz, ojos mantenimiento	span maintenance
movimiento lateral	lateral spread
	MTC (Ministry of Transportation & Communication
Comunicaciones) muro de contención	
neblina, niebla	retaining wall
	fog water level
nivel de agua	
pantalla	wingwall
pavimento	pavement

Español	Inglesa
pie (pies) (1 metro ~ 3.1 feet)	foot (feet)
pilar	pier / bent
pilote	pile
plano	flat
playa	beach
pontón (in Perú,>3m but <10m)	culvert
provincia	county
proyecto de construcción	construction project
puente (in Perú, >10m)	bridge
puente colapsado	collapsed bridge
ravine	pongo, barranco, hendidura profunda
relleno	fill
réplica	aftershock
rio	river
roca	rock
ruta	route
sección transversal	cross section
SIG (Sistema de Información Geográfica)	GIS (Geographic Information System)
Sistema de Posicionamiento Global	GPS (global positioning system)
sobrecarga	overweight
socavación	undermining
suelo (terreno)	soil (ground)
suelos blandos	soft soil (ground)
superficie del terreno	ground level
talud	slope
temblor	tremor
tenga cuidado	be careful
terremoto / sismo	earthquake
tienda /almace'n	store / shop
velocidad límite	speed limit
via, carretera	road
viga	girder
volcancito	sand boil / sand cone

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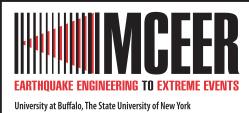
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